

T-shaped gravity wall design - EN 1997

This document shows the example of T-shaped wall design calculation according to Eurocode 7. The design situation is based on Eurocode 7: Geotechnical Design Worked examples ANNEX A.4 and involves T-shaped gravity wall with that is required to support granular fill. The ground and the fill are both dry.

References:

- [Eurocode 7: Geotechnical Design Worked examples](#)
- [EN 1997](#)

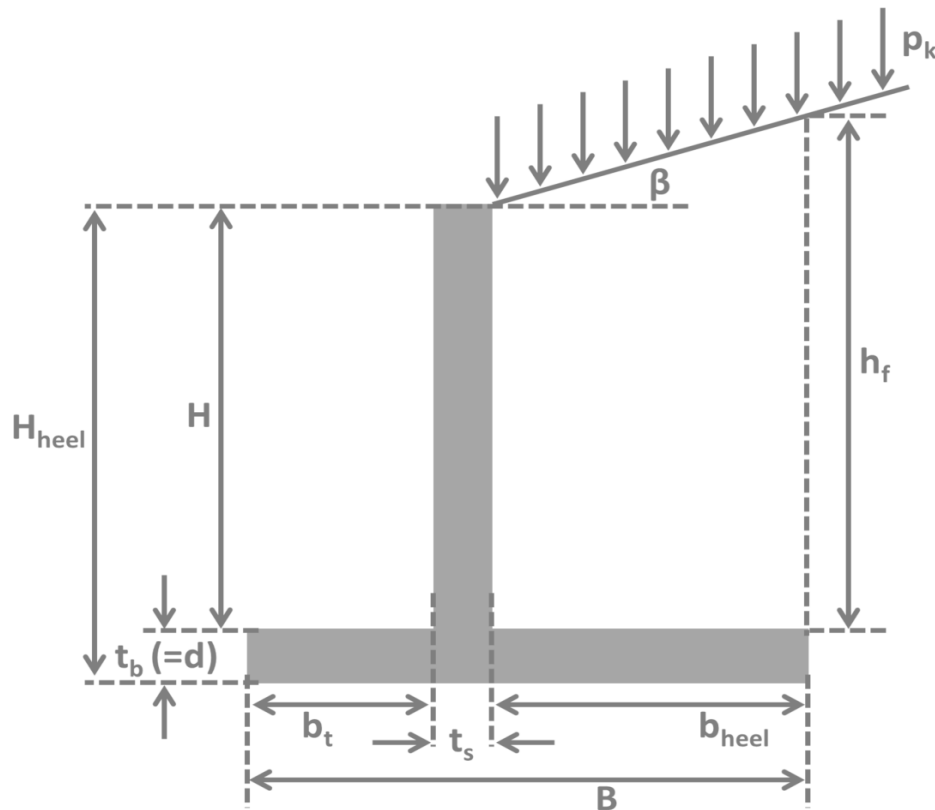


Figure 1 : T-shaped gravity wall with notation

1. Design parameters and conditions

Geometry

| | |
|--|--|
| Retained height | $H := 6.0 \text{ m}$ |
| Overall breadth | $B := 3.9 \text{ m}$ |
| Base thickness | $t_b := 0.8 \text{ m}$ |
| Toe width | $b_t := 0.95 \text{ m}$ |
| Thickness of wall stem | $t_s := 0.7 \text{ m}$ |
| Angle to the horizontal | $\beta := 20 \text{ deg}$ |
| Width wall heel | $b_{\text{heel}} := B - b_t - t_s = 2.250 \text{ m}$ |
| Height of fill about wall heel | $h_f := H + b_{\text{heel}} \cdot \tan(\beta) = 6.819 \text{ m}$ |
| Height of wall about wall heel include thickness o base | $H_{\text{heel}} := h_f + t_b = 7.619 \text{ m}$ |
| Depth of base of footing | $d := t_b = 0.800 \text{ m}$ |

Support granular fill

| | |
|--|---|
| Characteristic weight density | $\gamma_k := 19 \frac{\text{kN}}{\text{m}^3}$ |
| Cohesion intercept | $\phi_k := 32.5 \text{ deg}$ |
| Angle of shearing resistance in terms of effective stress | $c_k := 0 \text{ kPa}$ |
| Variable surcharge | $p_k := 5 \text{ kPa}$ |

Others

| | |
|--|---|
| Weight density of reinforced concrete | $\gamma_{c,k} := 25 \frac{\text{kN}}{\text{m}^3}$ |
|--|---|

2. Verification of drained strength (Geotechnical limit state)

Use DA1 : Design Approach 1

Combination 1 : A1, M1, R1

Combination 2 : A2, M2, R1

2-1. Material properties

Partial factors ([M1, M2])

Angle of shearing resistance $\gamma_\phi := [1, 1.25]$
Effective cohesion

$$\gamma_c := [1, 1.25]$$

Design angle of shearing resistance

$$\phi_d := \left[\text{seq} \left(\arctan \left(\frac{\tan(\phi_k)}{\gamma_\phi[i]} \right) \cdot \text{rad}, i = 1 \dots 2 \right) \right]$$

$$\phi_d = [32.500 \text{ arcdeg}, 27.006 \text{ arcdeg}]$$

Design effective cohesion

$$c_d := \left[\text{seq} \left(\frac{c_k}{\gamma_c[i]}, i = 1 \dots 2 \right) \right] = [0, 0.]$$

2-2. Actions

Partial factors ([A1, A2])

Permanent, Unfavourable $\gamma_G := [1.35, 1]$

Variable, Unfavourable $\gamma_Q := [1.5, 1.3]$

Characteristic self-weight of wall (Permanent action)

$$\text{wall stem} \quad W_{\text{stem_Gk}} := \gamma_{c,k} \cdot t_s \cdot H = 105.000 \frac{\text{kN}}{\text{m}}$$

$$\text{wall base} \quad W_{\text{base_Gk}} := \gamma_{c,k} \cdot t_b \cdot B = 78.000 \frac{\text{kN}}{\text{m}}$$

Characteristic total self-weight of wall

$$W_{\text{wall_Gk}} := W_{\text{stem_Gk}} + W_{\text{base_Gk}} = 183.000 \frac{\text{kN}}{\text{m}}$$

Characteristic total self-weight of backfill

$$W_{\text{fill_Gk}} := \gamma_k \cdot b_{\text{heel}} \cdot \frac{H + h_f}{2} = 274.005 \frac{\text{kN}}{\text{m}}$$

Characteristic total self-weight of wall and backfill

$$W_{\text{Gk}} := W_{\text{wall_Gk}} + W_{\text{fill_Gk}} = 457.005 \frac{\text{kN}}{\text{m}}$$

Characteristic surcharge (variable)

$$q_{\text{Qk}} := p_k = 5 \text{ kPa}$$

$$Q_{\text{Qk}} := q_{\text{Qk}} \cdot b_{\text{heel}} = 11.250 \frac{\text{kN}}{\text{m}}$$

Design active earth pressure coefficient

$$K_{a\beta_d} := \left[\text{seq} \left(\left(\frac{\cos(\beta) - \sqrt{\cos(\beta)^2 - \cos(\phi_d[i])^2}}{\cos(\beta) + \sqrt{\cos(\beta)^2 - \cos(\phi_d[i])^2}} \right) \cdot \cos(\beta), i = 1..2 \right) \right]$$

$$K_{a\beta_d} = [0.365, 0.486]$$

Equivalent coefficient to calculate horizontal thrust

$$K_{ah_d} := \left[\text{seq} (K_{a\beta_d}[i] \cdot \cos(\beta), i = 1..2) \right] = [0.343, 0.457]$$

Design thrust from earth pressure on back of virtual plane
(inclined at angle β to horizontal)

$$\text{From ground} \quad E_{a_Gd} := \left[\text{seq} \left(\gamma_G[i] \cdot K_{a\beta_d}[i] \cdot \left(\frac{1}{2} \cdot \gamma_k \cdot H_{\text{heel}}^2 \right), i = 1 \dots 2 \right) \right]$$

$$E_{a_Gd} = \left[271.396 \frac{\text{kN}}{\text{m}}, 268.229 \frac{\text{kN}}{\text{m}} \right]$$

$$\text{From surcharge} \quad E_{a_Qd} := \left[\text{seq} \left(\gamma_Q[i] \cdot K_{a\beta_d}[i] \cdot q_{Qk} \cdot H_{\text{heel}}, i = 1 \dots 2 \right) \right]$$

$$E_{a_Qd} = \left[20.831 \frac{\text{kN}}{\text{m}}, 24.088 \frac{\text{kN}}{\text{m}} \right]$$

$$\text{Total} \quad E_{a_d} := \left[\text{seq} \left(E_{a_Gd}[i] + E_{a_Qd}[i], i = 1 \dots 2 \right) \right]$$

$$E_{a_d} = \left[292.228 \frac{\text{kN}}{\text{m}}, 292.317 \frac{\text{kN}}{\text{m}} \right]$$

Horizontal design thrust

$$H_{Ed} := E_{a_d} \cdot \cos(\beta)$$

$$H_{Ed} = \left[274.604 \frac{\text{kN}}{\text{m}}, 274.689 \frac{\text{kN}}{\text{m}} \right]$$

Vertical(normal) design weight and thrust

$$N_{Ed} := \left[\text{seq} \left(\gamma_G[i] \cdot W_{Gk} + E_{a_d}[i] \cdot \sin(\beta), i = 1 \dots 2 \right) \right]$$

$$N_{Ed} = \left[716.904 \frac{\text{kN}}{\text{m}}, 556.983 \frac{\text{kN}}{\text{m}} \right]$$

2-3. Moments at wall toe - Bearing design situation

Partial factors (same value for both A1 and A2)

Permanent, favourable $\gamma_{G_{fav}} := 1.0$

Design overturning moments at wall toe

$$\begin{aligned} \text{From ground} \quad M_{Gd} &:= \left[\text{seq} \left(E_{a_{Gd}}[i] \cdot \left(\frac{1}{3} \cdot H_{heel} \cdot \cos(\beta) - B \cdot \sin(\beta) \right), i = 1 \dots 2 \right) \right] \\ M_{Gd} &= [285.674 \text{ kN}, 282.340 \text{ kN}] \end{aligned}$$

$$\begin{aligned} \text{From surcharge} \quad M_{Qd} &:= \left[\text{seq} \left(E_{a_{Qd}}[i] \cdot \left(\frac{1}{3} \cdot H_{heel} \cdot \cos(\beta) - B \cdot \sin(\beta) \right), i = 1 \dots 2 \right) \right] \\ M_{Qd} &= [21.927 \text{ kN}, 25.355 \text{ kN}] \end{aligned}$$

$$\begin{aligned} \text{Total} \quad M_{dst_d} &:= \left[\text{seq} \left(M_{Gd}[i] + M_{Qd}[i], i = 1 \dots 2 \right) \right] \\ M_{dst_d} &= [307.601 \text{ kN}, 307.695 \text{ kN}] \end{aligned}$$

Design restoring moments at wall toe

$$\text{From wall stem} \quad M_{stem_{Gd}} := \gamma_{G_{fav}} \cdot W_{stem_{Gk}} \cdot \left(b_t + \frac{t_s}{2} \right) = 136.500 \text{ kN}$$

$$\text{From wall base} \quad M_{base_{Gd}} := \gamma_{G_{fav}} \cdot W_{base_{Gk}} \cdot \frac{B}{2} = 152.100 \text{ kN}$$

$$\begin{aligned} \text{From backfill} \quad M_{fill_{Gd}} &:= \gamma_{G_{fav}} \cdot \gamma_k \cdot b_{heel} \cdot \left(H \cdot \left(B - \frac{b_{heel}}{2} \right) + \frac{(h_f - H)}{2} \cdot \left(B - \frac{b_{heel}}{3} \right) \right) \\ M_{fill_{Gd}} &= 766.927 \text{ kN} \end{aligned}$$

$$\text{From surcharge} \quad M_{Qd_{fav}} := \gamma_{G_{fav}} \cdot Q_{Qk} \cdot \frac{b_{heel}}{2} = 12.656 \text{ kN}$$

Total

$$M_{\text{stb}_d} := M_{\text{stem}_Gd} + M_{\text{base}_Gd} + M_{\text{fill}_Gd} + M_{\text{Qd}_fav}$$

$$M_{\text{stb}_d} = 1068.184 \text{ kN}$$

Line of action of resultant force

$$x := \left[\text{seq} \left(\frac{M_{\text{stb}_d} - M_{\text{dst}_d}[i]}{N_{\text{Ed}}[i]}, i = 1 .. 2 \right) \right] = [1.061 \text{ m}, 1.365 \text{ m}]$$

Eccentricity of actions from center line of base

$$e_d := \left[\text{seq} \left(\frac{B}{2} - x[i], i = 1 .. 2 \right) \right] = [0.889 \text{ m}, 0.585 \text{ m}]$$

Effective width of base

$$B_d := \left[\text{seq} (B - 2 \cdot e_d[i], i = 1 .. 2) \right] = [2.122 \text{ m}, 2.731 \text{ m}]$$

2-4. Bearing resistance

Design bearing capacity factors

$$N_{q_d} := \left[\text{seq} \left(e^{\pi \cdot \tan(\phi_d[i])} \cdot \left(\tan \left(45 \cdot \text{deg} + \frac{\phi_d[i]}{2} \right) \right)^2, i = 1 .. 2 \right) \right] = [24.585, 13.208]$$

$$N_{\gamma_d} := \left[\text{seq} (2 \cdot (N_{q_d}[i] - 1) \cdot \tan(\phi_d[i]), i = 1 .. 2) \right] = [30.050, 12.443]$$

Shape factors (for an infinity long footing)

$$s_q := 1.0$$

$$s_\gamma := 1.0$$

Inclination factors (for an infinity long footing)

$$m_R := 2$$

$$i_q := \left[\text{seq} \left(\left(1 - \frac{H_{Ed}[i]}{N_{Ed}[i] + A \cdot c_d[i] \cdot \cot(\phi_d[i])} \right)^{m_B}, i = 1 \dots 2 \right) \right] = [0.381, 0.257]$$

$$i_\gamma := \left[\text{seq} \left(i_q[i]^{\frac{m_B + 1}{m_B}}, i = 1 \dots 2 \right) \right] = [0.235, 0.130]$$

Partial factors (R1)

Bearing $\gamma_{Rv} := 1$

Design bearing resistance

From overburden $q_{Rvq,d} := \left[\text{seq} \left(\frac{\gamma_k \cdot d \cdot N_{q,d}[i] \cdot s_q \cdot i_q[i]}{\gamma_{Rv}}, i = 1 \dots 2 \right) \right]$

$$q_{Rvq,d} = [142.239 \text{ kPa}, 51.569 \text{ kPa}]$$

From body-mass $q_{Rv\gamma,d} := \left[\text{seq} \left(\frac{1}{2} \cdot \left(\frac{B_d[i] \cdot \gamma_k \cdot N_{\gamma,d}[i] \cdot s_\gamma \cdot i_\gamma[i]}{\gamma_{Rv}} \right), i = 1 \dots 2 \right) \right]$

$$q_{Rv\gamma,d} = [142.250 \text{ kPa}, 42.027 \text{ kPa}]$$

Total $q_{Rv,d} := \left[\text{seq} (q_{Rvq,d}[i] + q_{Rv\gamma,d}[i], i = 1 \dots 2) \right]$

$$q_{Rv,d} = [284.488 \text{ kPa}, 93.596 \text{ kPa}]$$

Characteristic bearing resistance

$$N_{Rd} := \left[\text{seq} (q_{Rv,d}[i] \cdot B_d[i], i = 1 \dots 2) \right] = \left[603.643 \frac{\text{kN}}{\text{m}}, 255.586 \frac{\text{kN}}{\text{m}} \right]$$

3. Verifications

3-1. Verification of resistance to sliding

Partial factors (R1)

Sliding $\gamma_{Rh} := 1$

Interface friction angle

$k := 1$ $\phi_{cv,k} := 30 \text{ deg}$ $\sigma_d := k \cdot \phi_{cv,k} = 30 \text{ arcdeg}$

Design sliding resistance

(ignoring adhesion, as required by EN 1997-1 exp 6.3 a)

$$H_{Rd} := \left[\text{seq} \left(\frac{\gamma_{G_{fav}} \cdot N_{Ed}[i] \cdot \tan(\sigma_d)}{\gamma_{Rh}}, i = 1 \dots 2 \right) \right] = \left[413.905 \frac{\text{kN}}{\text{m}}, 321.574 \frac{\text{kN}}{\text{m}} \right]$$

Overdesign factor

Calculation $ODF := \left[\text{seq} \left(\frac{H_{Rd}[i]}{H_{Ed}[i]}, i = 1 \dots 2 \right) \right] = [1.507, 1.171]$

Check $\left[\text{seq} \left(\left\{ \begin{array}{ll} \text{"OK"} & ODF[i] \geq 1 \\ \text{"NG"} & \text{otherwise} \end{array} \right. , i = 1 \dots 2 \right) \right] = [\text{"OK"}, \text{"OK"}]$

Degree of utilization

Calculation $\Lambda := \left[\text{seq} \left(\frac{H_{Ed}[i]}{H_{Rd}[i]}, i = 1 \dots 2 \right) \right] = [66.34\%, 85.42\%]$

Check $\left[\text{seq} \left(\left\{ \begin{array}{ll} \text{"OK"} & \Lambda[i] \leq 1 \\ \text{"NG"} & \text{otherwise} \end{array} \right. , i = 1 \dots 2 \right) \right] = [\text{"OK"}, \text{"OK"}]$

3-2. Verification of bearing resistance

Design bearing resistance

$$N_{Rd} = \left[603.643 \frac{\text{kN}}{\text{m}}, 255.586 \frac{\text{kN}}{\text{m}} \right]$$

Design bearing force

$$N_{Ed} = \left[716.904 \frac{\text{kN}}{\text{m}}, 556.983 \frac{\text{kN}}{\text{m}} \right]$$

Overdesign factor

Calculation

$$\text{ODF} := \left[\text{seq} \left(\frac{N_{Rd}[i]}{N_{Ed}[i]}, i = 1 \dots 2 \right) \right] = [0.842, 0.459]$$

Check

$$\left[\text{seq} \left(\left\{ \begin{array}{ll} \text{"OK"} & \text{ODF}[i] < 1 \\ \text{"NG"} & \text{otherwise} \end{array} \right. , i = 1 \dots 2 \right) \right] = [\text{"OK"}, \text{"OK"}]$$

Degree of utilization

Calculation

$$\Lambda := \left[\text{seq} \left(\frac{N_{Ed}[i]}{N_{Rd}[i]}, i = 1 \dots 2 \right) \right] = [118.76\%, 217.92\%]$$

Check

$$\left[\text{seq} \left(\left\{ \begin{array}{ll} \text{"OK"} & \Lambda[i] > 1 \\ \text{"NG"} & \text{otherwise} \end{array} \right. , i = 1 \dots 2 \right) \right] = [\text{"OK"}, \text{"OK"}]$$